

Gabion analysis

Input data

Project

Date : 11/2/2005

Material of blocks - filling

No.	Name	γ [kN/m ³]	ϕ [°]	c [kPa]
1	Material No. 1	17.00	30.00	0.00

Material of blocks - mesh

No.	Name	Strength overh. R_t [kN/m]	Spacing of vert. meshes b [m]	Bear.cap. of front joint R_s [kN/m]
1	Material No. 1	40.00	1.00	40.00

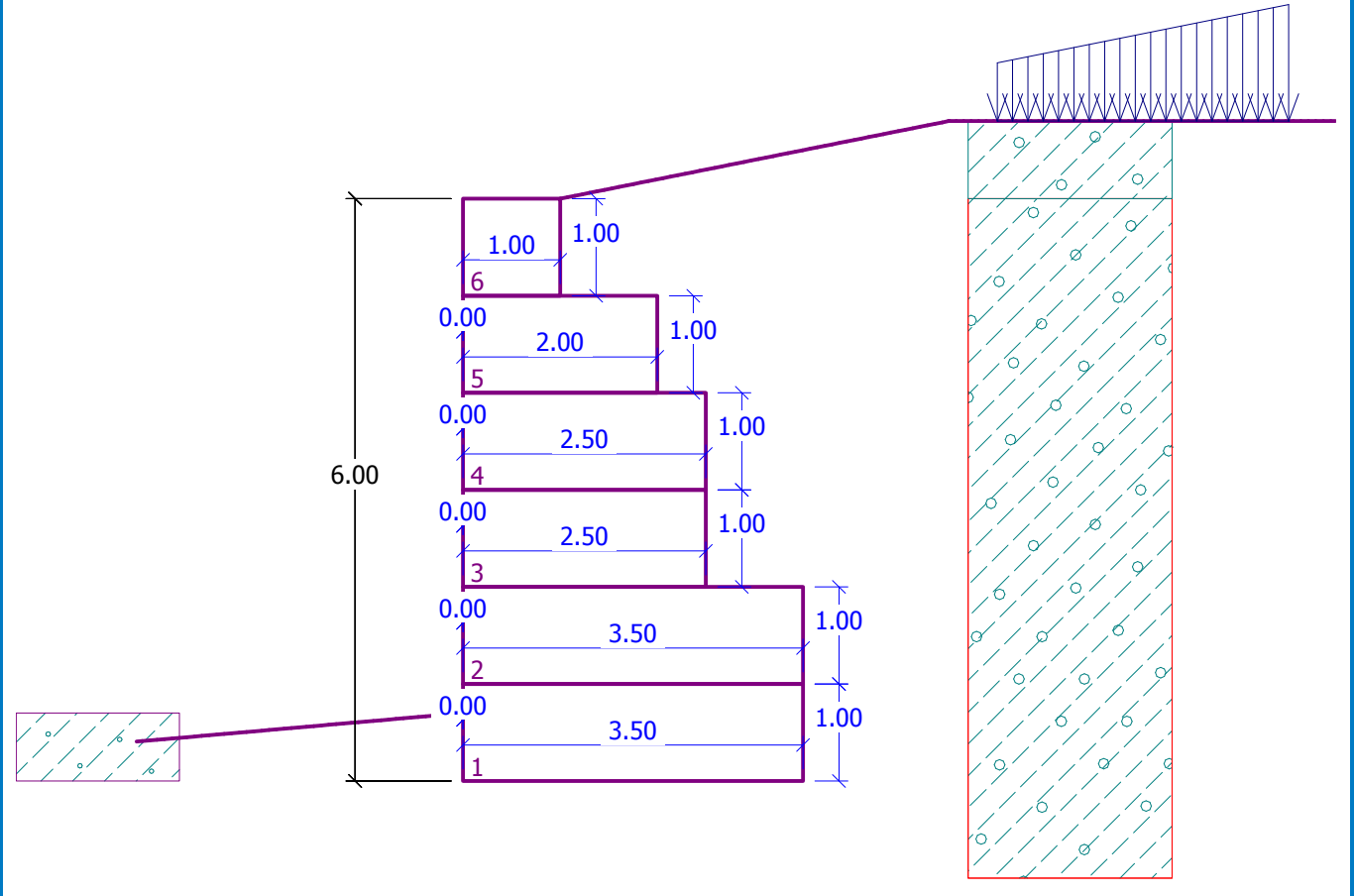
Geometry of structure

No.	Width b [m]	Height h [m]	Offset a [m]	Material
6	1.00	1.00	0.00	Material No. 1
5	2.00	1.00	0.00	Material No. 1
4	2.50	1.00	0.00	Material No. 1
3	2.50	1.00	0.00	Material No. 1
2	3.50	1.00	0.00	Material No. 1
1	3.50	1.00	-	Material No. 1

Gabion slope = 0.00 °
Overall height = 6.00 m
Overall wall volume = 15.00 m³/m

Name : Geometry

Stage : 1



Soil parameters


Soil No. 1

Unit weight : $\gamma = 20.00 \text{ kN/m}^3$
 Stress-state : effective
 Angle of internal friction : $\varphi_{ef} = 25.00^\circ$
 Cohesion of soil : $c_{ef} = 9.00 \text{ kPa}$
 Angle of friction struc.-soil : $\delta = 16.00^\circ$
 Soil : cohesionless
 Solid unit weight : $\gamma_s = 25.00 \text{ kN/m}^3$
 Porosity <0.0 - 1.0> : $n = 0.30$

Soil No. 2

Unit weight : $\gamma = 19.00 \text{ kN/m}^3$
 Stress-state : effective
 Angle of internal friction : $\varphi_{ef} = 30.00^\circ$
 Cohesion of soil : $c_{ef} = 5.00 \text{ kPa}$
 Angle of friction struc.-soil : $\delta = 16.00^\circ$
 Soil : cohesionless
 Saturated unit weight : $\gamma_{sat} = 20.00 \text{ kN/m}^3$

Geological profile and assigned soils

No.	Layer [m]	Assigned soil	Pattern
1	-	Soil No. 1	

Terrain profile

Terrain behind construction has the slope 1: 5.00 (slope angle is 11.31 °).
Embankment height is 0.80 m, embankment length is 4.00 m.

Water influence

Ground water table is located below the structure.

Inserted surface loads

No.	Surcharge		Name	Action	Mag.1 [kN/m ²]	Mag.2 [kN/m ²]	Ord.x x [m]	Length l [m]	Depth z [m]
	new	change							
1	YES		Surcharge No. 1	permanent	10.00	20.00	4.50	3.00	on terrain

Resistance on front face of the structure

Resistance on front face of the structure: 2/3 pass., 1/3 at rest

Soil on front face of the structure - Soil No. 2

Soil thickness in front of structure h = 0.70 m

Angle of friction struc.-soil $\delta = 0.00^\circ$

Soil slope in front of structure $\beta = -5.00^\circ$

Global settings

Verification methodology : Analysis according to EN 1997

Input of partial factors : Standard

Design approach : 1 - reduction of actions and materials

Partial factors on actions (F)	Fact.	Combination 1 [-]		Combination 2 [-]	
		Unfavourable	Favourable	Unfavourable	Favourable
Permanent actions	γ_G	1.35	1.00	1.00	1.00
Variable actions	γ_Q	1.50	0.00	1.30	0.00
Water actions	γ_w	1.30		1.00	

Partial factors for soil parameters (M)	Fact.	Combination 1 [-]	Combination 2 [-]
Partial factor for internal friction	$\gamma_{m\phi}$	1.00	1.25
Partial factor for effective cohesion	γ_{mc}	1.00	1.25
Partial factor for undrained shear strength	$\gamma_{m_{cu}}$	1.00	1.40
Partial factor for Poisson's ratio	γ_{mv}	1.00	1.00

Partial factors for variable actions	Fact.	[-]
Factor for combination value	ψ_0	0.70
Factor for frequent value	ψ_1	0.50
Factor for quasi-permanent value	ψ_2	0.30

Active earth pressure calculation - Coulomb

Passive earth pressure calculation - Caquot-Kerisel

Settings of the stage of construction

Combination : basic

Verification No. 1

Forces acting on construction - combination 1

Name	F _{hor} [kN/m]	App.Pt. Z [m]	F _{vert} [kN/m]	App.Pt. X [m]	Coeff. overtur.	Coeff. sliding	Coeff. stress
Weight - wall	0.00	-2.43	255.00	1.40	1.000	1.000	1.350
FF resistance	-15.76	-0.29	0.00	0.00	1.000	1.000	1.000
Weight - earth wedge	0.00	-3.95	40.99	2.20	1.000	1.000	1.350

Name	F _{hor} [kN/m]	App.Pt. Z [m]	F _{vert} [kN/m]	App.Pt. X [m]	Coeff. overtur.	Coeff. sliding	Coeff. stress
Active pressure	126.39	-2.07	97.75	3.01	1.000	1.350	1.350
Surcharge No. 1	10.74	-2.14	8.07	2.51	1.350	1.350	1.350

Verification of complete wall

Check for overturning stability

Resisting moment $M_{res} = 769.01$ kNm/m

Overturning moment $M_{ovr} = 287.93$ kNm/m

Wall for overturning is SATISFACTORY

Check for slip

Resisting horizontal force $H_{res} = 224.85$ kN/m

Active horizontal force $H_{act} = 169.37$ kN/m

Wall for slip is SATISFACTORY

Overall check - WALL is SATISFACTORY

Maximum stress in footing bottom :226.63kPa

Forces acting on construction - combination 2

Name	F _{hor} [kN/m]	App.Pt. Z [m]	F _{vert} [kN/m]	App.Pt. X [m]	Coeff. overtur.	Coeff. sliding	Coeff. stress
Weight - wall	0.00	-2.43	255.00	1.40	1.000	1.000	1.000
FF resistance	-12.27	-0.28	0.00	0.00	1.000	1.000	1.000
Weight - earth wedge	0.00	-3.95	40.99	2.20	1.000	1.000	1.000
Active pressure	163.53	-2.09	106.01	2.99	1.000	1.000	1.000
Surcharge No. 1	13.84	-2.42	9.50	2.37	1.000	1.000	1.000

Verification of complete wall

Check for overturning stability

Resisting moment $M_{res} = 786.60$ kNm/m

Overturning moment $M_{ovr} = 371.18$ kNm/m

Wall for overturning is SATISFACTORY

Check for slip

Resisting horizontal force $H_{res} = 168.04$ kN/m

Active horizontal force $H_{act} = 165.10$ kN/m

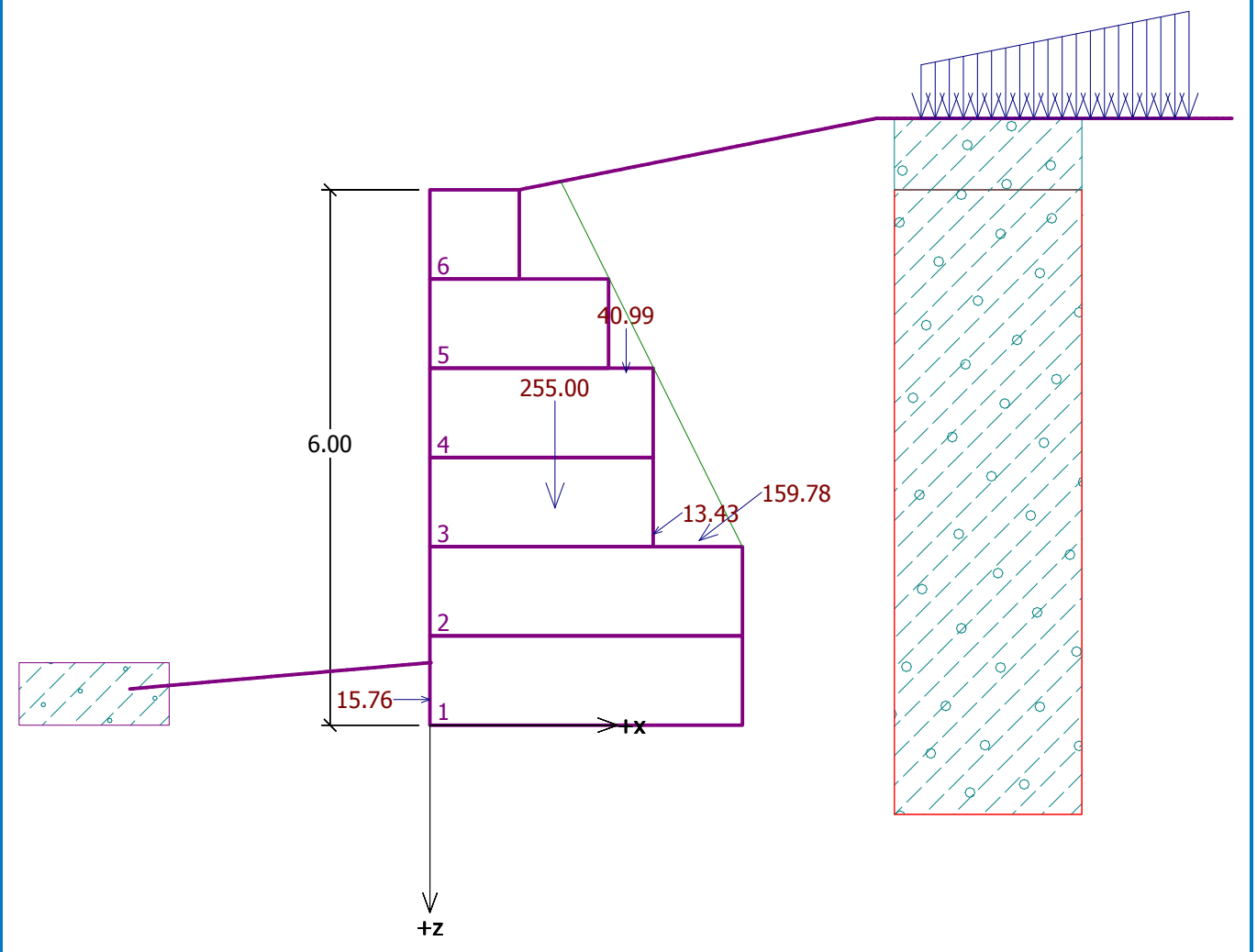
Wall for slip is SATISFACTORY

Overall check - WALL is SATISFACTORY

Maximum stress in footing bottom :203.80kPa

Name : Verification

Stage : 1; Analysis : 1



Bearing capacity of foundation soil

Forces acting at the centre of the footing bottom

Number	Moment [kNm/m]	Norm. force [kN/m]	Shear Force [kN/m]	Eccentricity [m]	Stress [kPa]
1	300.10	542.44	169.37	0.55	226.63

Bearing capacity of foundation soil check

Eccentricity verification

Max. eccentricity of normal force $e = 553.2$ mm

Maximum allowable eccentricity $e_{alw} = 1155.0$ mm

Eccentricity of the normal force is SATISFACTORY

Footing bottom bearing capacity verification

Max. stress at footing bottom $\sigma = 226.63$ kPa

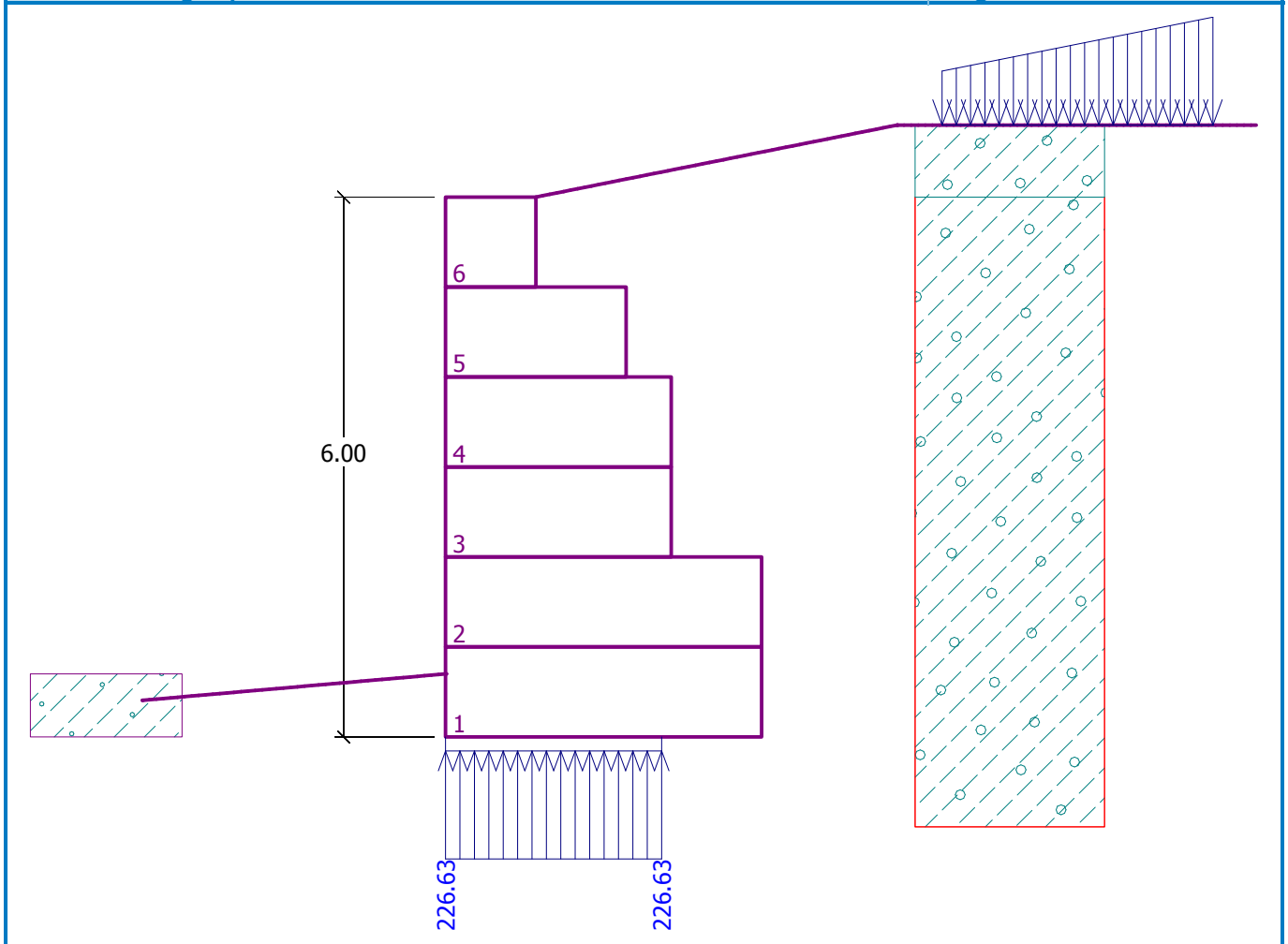
Bearing capacity of foundation soil $R_d = 230.00$ kPa

Bearing capacity of foundation soil is SATISFACTORY

Overall verification - bearing capacity of found. soil is SATISFACTORY

Name : Bearing cap.

Stage : 1



Dimensioning No. 1

Forces acting on construction - combination 1

Name	F _{hor} [kN/m]	App.Pt. Z [m]	F _{vert} [kN/m]	App.Pt. X [m]	Coeff. overtur.	Coeff. sliding	Coeff. stress
Weight - wall	0.00	-2.02	195.50	1.29	1.000	1.000	1.350
Weight - earth wedge	0.00	-2.95	40.99	2.20	1.000	1.000	1.350
Active pressure	91.41	-1.68	87.72	2.96	1.000	1.350	1.350
Surcharge No. 1	8.12	-1.66	7.32	2.45	1.000	1.350	1.350

Verification of construction joint above the block No.: 1

Check for overturning stability

Resisting moment $M_{res} = 620.42$ kNm/m

Overturning moment $M_{ovr} = 166.64$ kNm/m

Joint for overturning stability is SATISFACTORY

Check for slip

Resisting horizontal force $H_{res} = 210.61$ kN/m

Active horizontal force $H_{act} = 134.37$ kN/m

Joint for slip is SATISFACTORY

Forces acting on construction - combination 2

Name	F _{hor} [kN/m]	App.Pt. Z [m]	F _{vert} [kN/m]	App.Pt. X [m]	Coeff. overtur.	Coeff. sliding	Coeff. stress
Weight - wall	0.00	-2.02	195.50	1.29	1.000	1.000	1.000
Weight - earth wedge	0.00	-2.95	40.99	2.20	1.000	1.000	1.000
Active pressure	118.26	-1.70	95.49	2.93	1.000	1.000	1.000
Surcharge No. 1	10.87	-1.94	8.80	2.32	1.000	1.000	1.000

Verification of construction joint above the block No.: 1

Check for overturning stability

Resisting moment $M_{res} = 643.53$ kNm/m

Overturning moment $M_{ovr} = 222.07$ kNm/m

Joint for overturning stability is SATISFACTORY

Check for slip

Resisting horizontal force $H_{res} = 196.75$ kN/m

Active horizontal force $H_{act} = 129.13$ kN/m

Joint for slip is SATISFACTORY

Verification of gabion block for maximum stress:

Maximum pressure on the bottom block = 163.49 kPa

Red.Coeff. by offset of top block = 1.00

Average value of pressure on face = 71.66 kPa

Shear force trasmitted by friction = 170.54 kN/m

Bearing capacity against transverse pressure:

Joint bear.capacity = 40.00 kN/m

Computed stress-state = 35.83 kN/m

Transverse pressure check is SATISFACTORY

Joint btw. blocks check:

Mesh material bear.capacity = 40.00 kN/m

Computed stress-state = 35.83 kN/m

Joint between blocks is SATISFACTORY

Name : Dimensioning

Stage : 1; Dimensioning : 1

