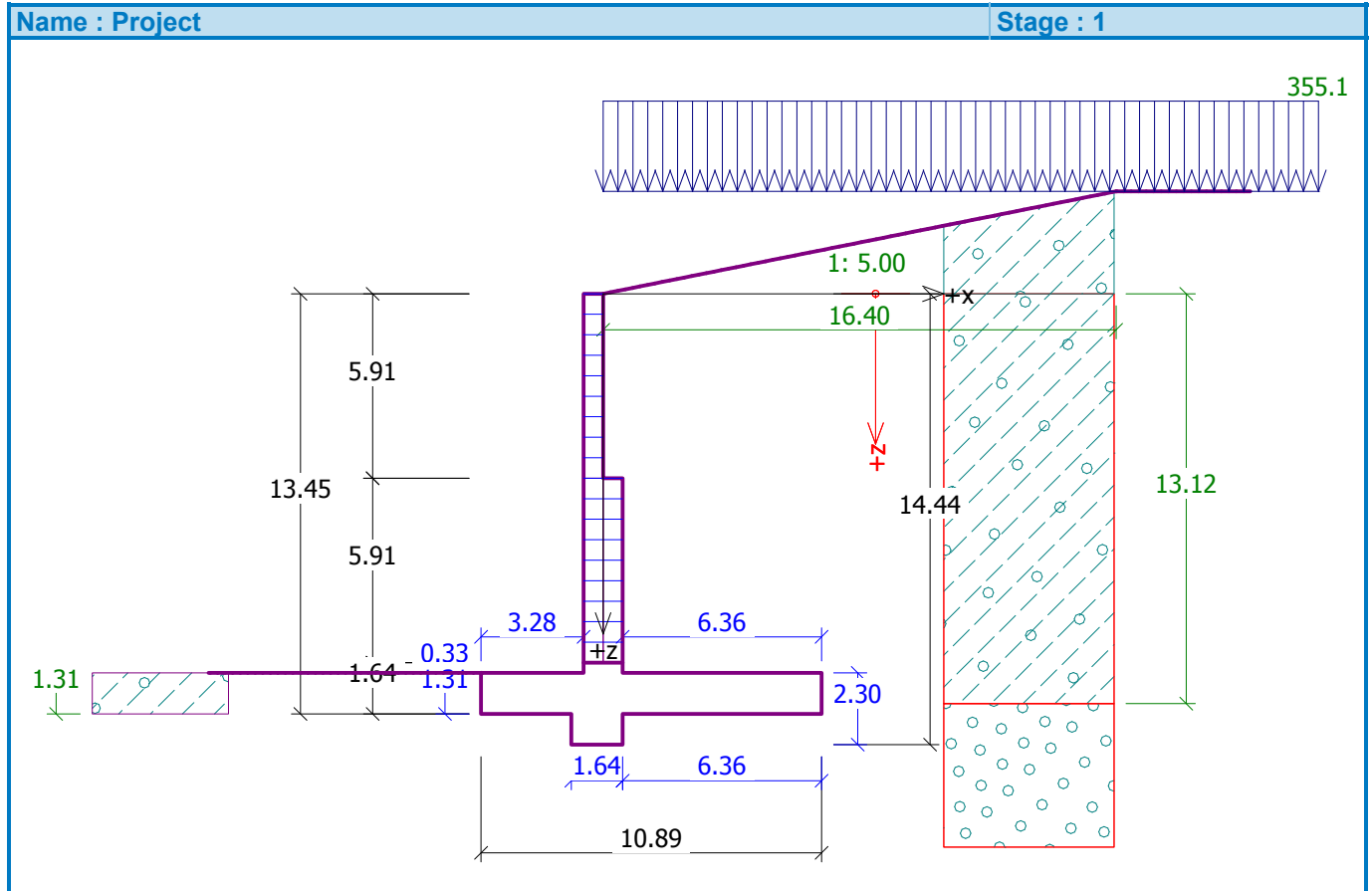


Verification of reinforced masonry wall

Input data

Project

Date : 10/2/2009



Material of structure

Unit weight $\gamma = 146.4$ pcf

Analysis of concrete structures carried out according to the standard EN 1992 1-1 (EC2).

Concrete : C 20/25

Cylinder compressive strength

$$f_{ck} = 2900.8 \text{ psi}$$

Tensile strength

$$f_{ct} = 319.1 \text{ psi}$$

Elasticity modulus

$$E_{cm} = 4206.1 \text{ ksi}$$

Longitudinal steel : B420

Yield strength

$$f_{yk} = 60915.8 \text{ psi}$$

Elasticity modulus

$$E = 29007.5 \text{ ksi}$$

Types of blocks

Number	Name block	Width b [ft]	Height h [ft]
1	140 x 200	0.46	0.66
2	190 x 200	0.62	0.66
3	290 x 200	0.95	0.66

Geometry

No.	Coordinate X [ft]	Depth Z [ft]
1	0.00	0.00

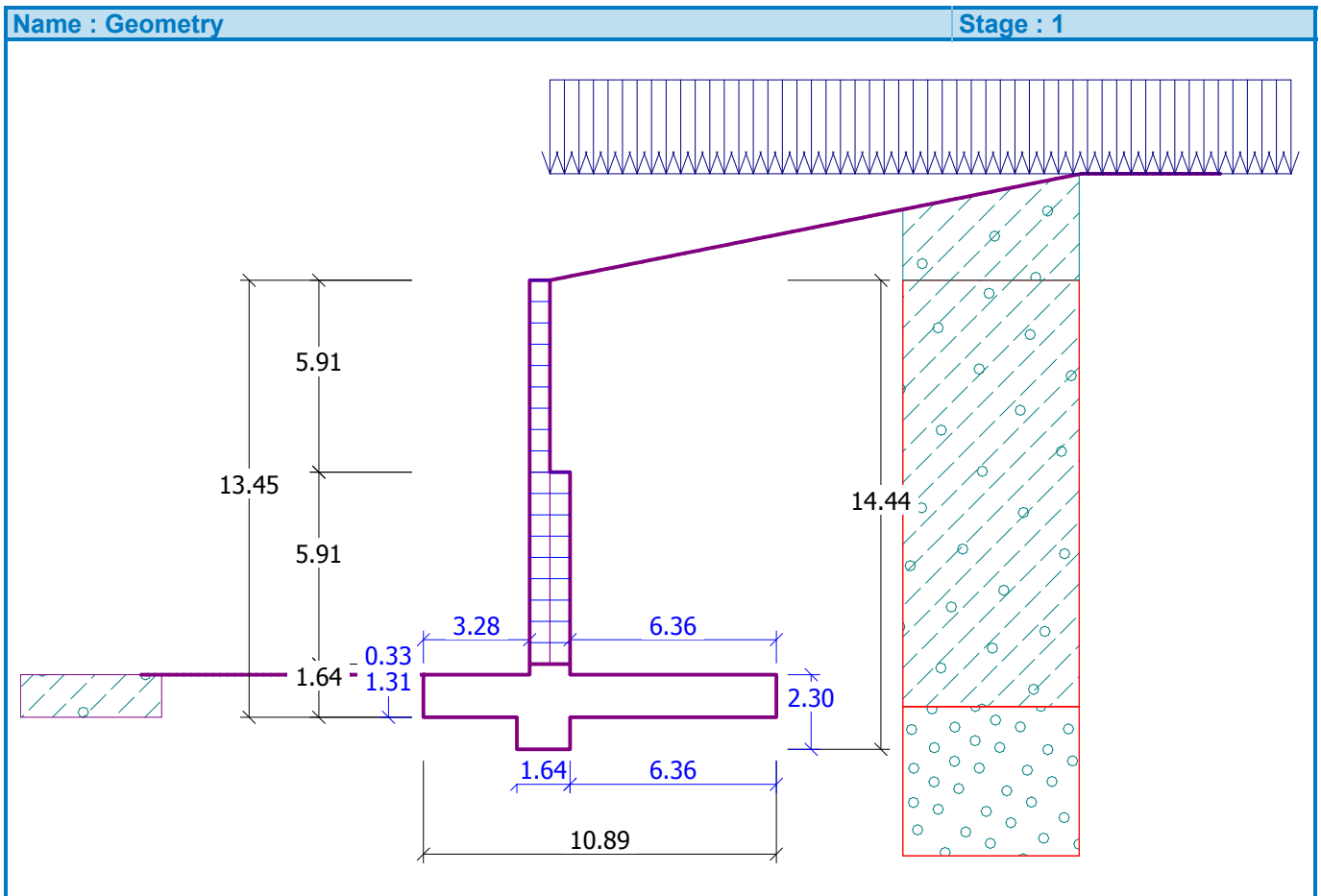
No.	Coordinate X [ft]	Depth Z [ft]
2	0.00	5.91
3	0.62	5.91
4	0.62	12.14
5	6.99	12.14
6	6.99	13.45
7	0.62	13.45
8	0.62	14.44
9	-1.02	14.44
10	-1.02	13.45
11	-3.90	13.45
12	-3.90	12.14
13	-0.62	12.14
14	-0.62	0.00

The origin [0,0] is located at the most upper right point of the wall.
 Wall section area = 27.36 ft².


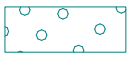
Geometry of masonry

Number of blocks in 1.row : 9 (type: 190 x 200)
 Number of blocks in 2. rowe : 9 (type: 190 x 200)
 Distance between blocks = 0.00 ft
 Number of blocks in upper part of wall: 9 (type: 190 x 200)

Characteristic compressive strength $f_k = 2175.6$ psi
 Characteristic shear strength $f_{vk} = 46.4$ psi



Basic soil parameters

No.	Name	Pattern	ϕ_{ef} [°]	C_{ef} [psf]	γ [pcf]	γ_{su} [pcf]	δ [°]
1	Soil No. 1		27.00	62.7	121.0	58.5	14.00
2	Soil No. 2		34.00	0.0	121.0	58.5	17.00

All soils are considered as cohesionless for at rest pressure analysis.

Soil parameters


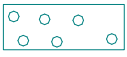
Soil No. 1

Unit weight : $\gamma = 121.0$ pcf
 Stress-state : effective
 Angle of internal friction : $\phi_{ef} = 27.00^\circ$
 Cohesion of soil : $C_{ef} = 62.7$ psf
 Angle of friction struc.-soil : $\delta = 14.00^\circ$
 Soil : cohesionless
 Saturated unit weight : $\gamma_{sat} = 121.0$ pcf

Soil No. 2

Unit weight : $\gamma = 121.0$ pcf
 Stress-state : effective
 Angle of internal friction : $\phi_{ef} = 34.00^\circ$
 Cohesion of soil : $C_{ef} = 0.0$ psf
 Angle of friction struc.-soil : $\delta = 17.00^\circ$
 Soil : cohesionless
 Saturated unit weight : $\gamma_{sat} = 121.0$ pcf

Geological profile and assigned soils

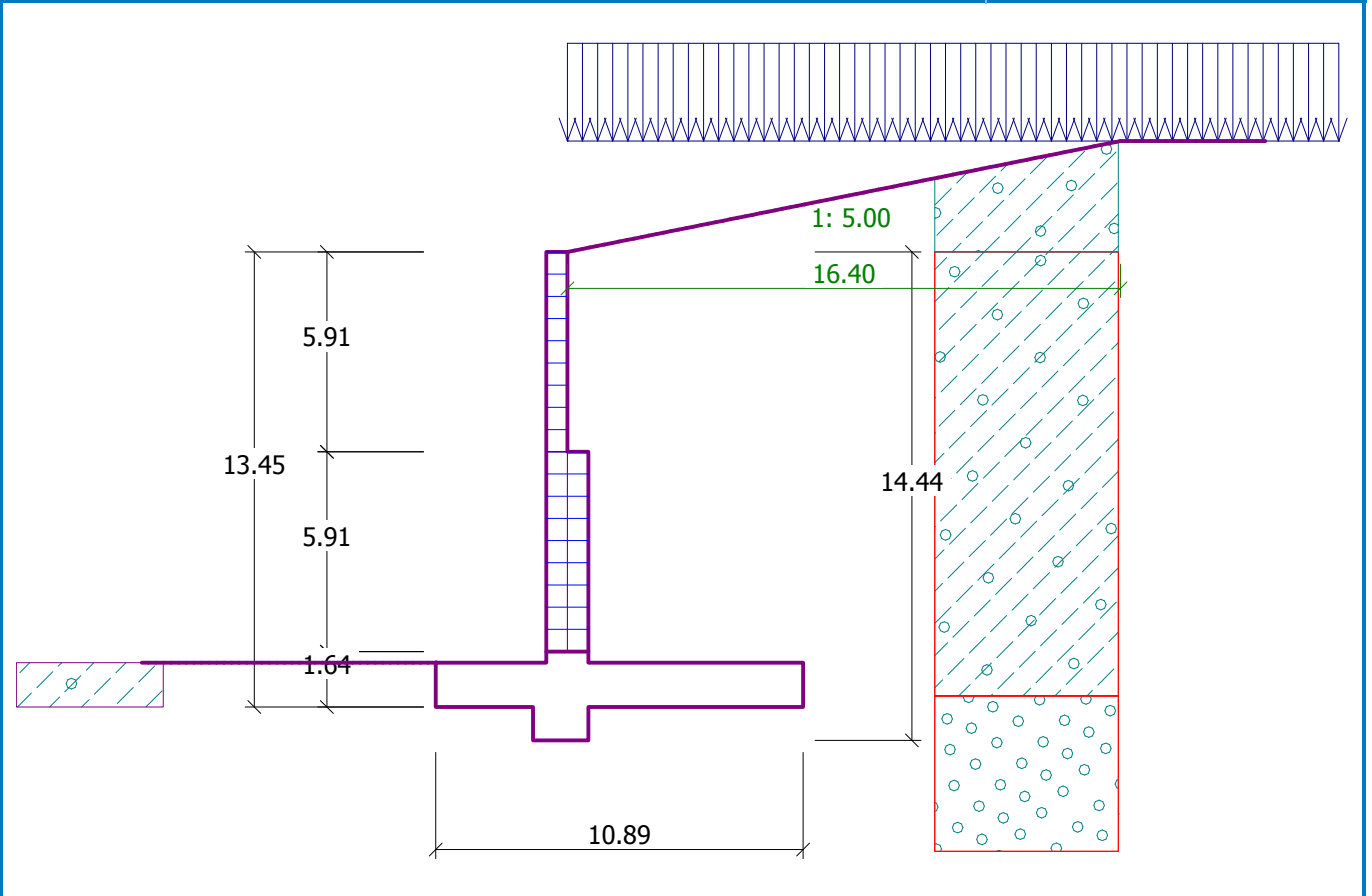
No.	Layer [ft]	Assigned soil	Pattern
1	13.12	Soil No. 1	
2	-	Soil No. 2	

Terrain profile

Terrain behind construction has the slope 1: 5.00 (slope angle is 11.31 °).
 Embankment height is 3.28 ft, embankment length is 16.40 ft.

Name : Terrain

Stage : 1



Water influence

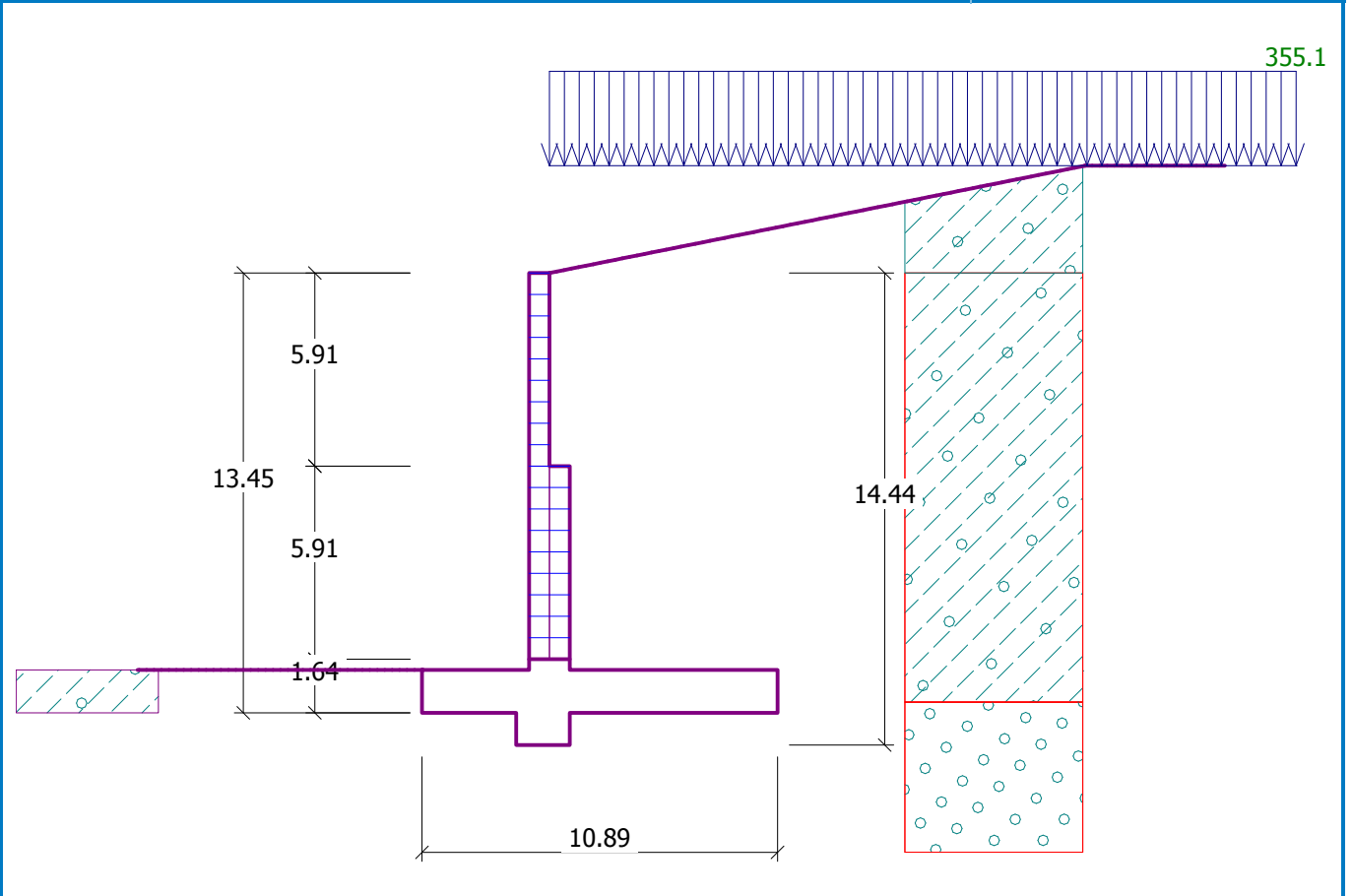
Ground water table is located below the structure.

Inserted surface loads

No.	Surcharge		Name	Action	Mag.1 [lb/ft ²]	Mag.2 [lb/ft ²]	Ord.x x [ft]	Length l [ft]	Depth z [ft]
	new	change							
1	YES		Surcharge No. 1	permanent	355.1				on terrain

Name : Surchage

Stage : 1



Resistance on front face of the structure

Resistance on front face of the structure: at rest
Soil on front face of the structure - Soil No. 1
Soil thickness in front of structure $h = 1.31$ ft
Terrain in front of structure is flat.

Global settings

Verification methodology : Classical way
Active earth pressure calculation - Coulomb
Passive earth pressure calculation - Caquot-Kerisel
Analysis standard for reinforced masonry - EN 1992 1-1
Standard for concrete structures - EN 1992 1-1 (EC2)

Settings of the stage of construction

Analysis carried out according to classical theory (safety factor)

Safety factor for slip = 1.50
Safety factor for overturning = 1.50
Factor of safety for bearing capacity = 1.00

The wall is free to move. Active earth pressure is therefore assumed.

Verification No. 1

Forces acting on construction

Name	F_{hor} [lbf/ft]	App.Pt. Z [ft]	F_{vert} [lbf/ft]	App.Pt. X [ft]	Design coefficient
Weight - wall	0.0	-2.98	4006.2	4.66	1.000

Name	F _{hor} [lb/ft]	App.Pt. Z [ft]	F _{vert} [lb/ft]	App.Pt. X [ft]	Design coefficient
FF resistance	-174.2	0.22	0.0	0.00	1.000
Weight - earth wedge	0.0	-6.24	5780.2	6.49	1.000
Active pressure	5060.0	-4.08	5212.3	9.05	1.000
Surcharge No. 1	2101.7	-6.89	2386.5	6.56	1.000

Verification of complete wall

Check for overturning stability

Resisting moment $M_{res} = 118989.7$ lbfft/ft
Overturning moment $M_{OVR} = 35145.0$ lbfft/ft

Safety factor = 3.39 > 1.50

Wall for overturning is SATISFACTORY

Check for slip

Resisting horizontal force $H_{res} = 11726.4$ lb/ft
Active horizontal force $H_{act} = 6987.6$ lb/ft

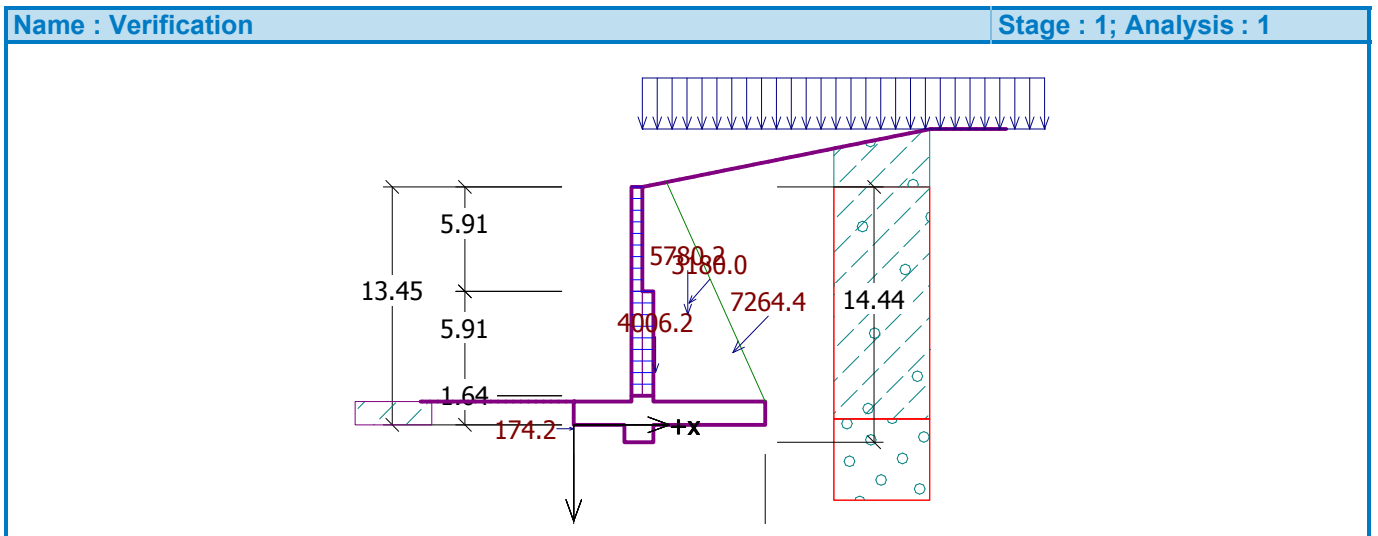
Safety factor = 1.68 > 1.50

Wall for slip is SATISFACTORY

Forces acting at the centre of footing bottom

Overall moment $M = 10838.2$ lbfft/ft
Normal force $N = 17385.2$ lb/ft
Shear force $Q = 6987.6$ lb/ft

Overall check - WALL is SATISFACTORY



Bearing capacity of foundation soil

Forces acting at the centre of the footing bottom

Number	Moment [lbfft/ft]	Norm. force [lb/ft]	Shear Force [lb/ft]	Eccentricity [ft]	Stress [psf]
1	10838.2	17385.2	6987.6	0.62	1802.4

Bearing capacity of foundation soil check

Eccentricity verification

Max. eccentricity of normal force $e = 7.48$ in

Maximum allowable eccentricity $e_{alw} = 43.13$ in

Eccentricity of the normal force is SATISFACTORY

Footing bottom bearing capacity verification

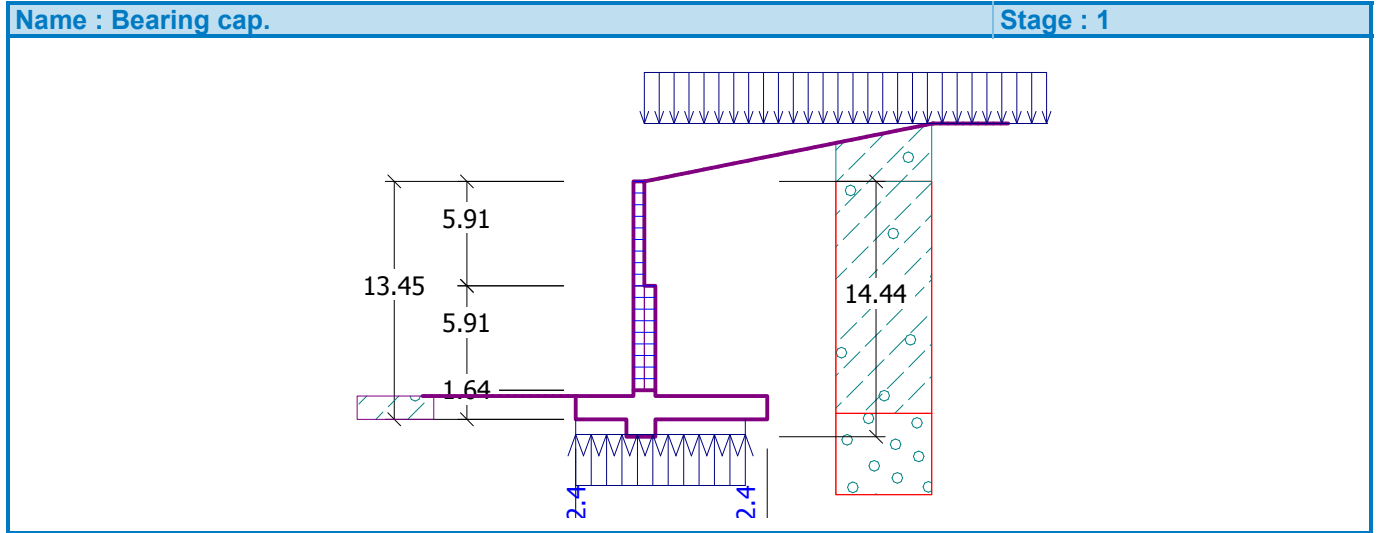
Max. stress at footing bottom $\sigma = 1802.4$ psf

Bearing capacity of foundation soil $R_d = 2506.3$ psf

Safety factor = 1.39 > 1.00

Bearing capacity of foundation soil is SATISFACTORY

Overall verification - bearing capacity of found. soil is SATISFACTORY



Dimensioning No. 1

Forces acting on construction

Name	F_{hor} [lbf/ft]	App.Pt. Z [ft]	F_{vert} [lbf/ft]	App.Pt. X [ft]	Design coefficient
Weight - wall	0.0	-4.92	1616.8	0.52	1.000
Weight - earth wedge	0.0	-6.36	51.9	0.83	1.000
Active pressure	2597.3	-3.57	1002.9	1.08	1.000
Surcharge No. 1	1610.2	-5.74	644.9	0.96	1.000

Verification of the joint, 38.75 ft from the top.

Back face reinforcement :

Diameter = 0.79 in

Spacing = 11.81 in

Cover = 1.18 in

There is no reinforcement on the front face.

Wall aspect ratio: 9.47

Verification of cross section in compression:

Ultimate normal force $N_{Rd} = 234522.6$ lbf/ft > 3316.5 lbf/ft = N_{Ed}

Cross section is SATISFACTORY

Verification of cross section in bending:

Ultimate bending moment $M_{Rd} = 28387.5$ lbfft/ft > 18002.0 lbfft/ft = M_{Ed}

Cross section is SATISFACTORY.

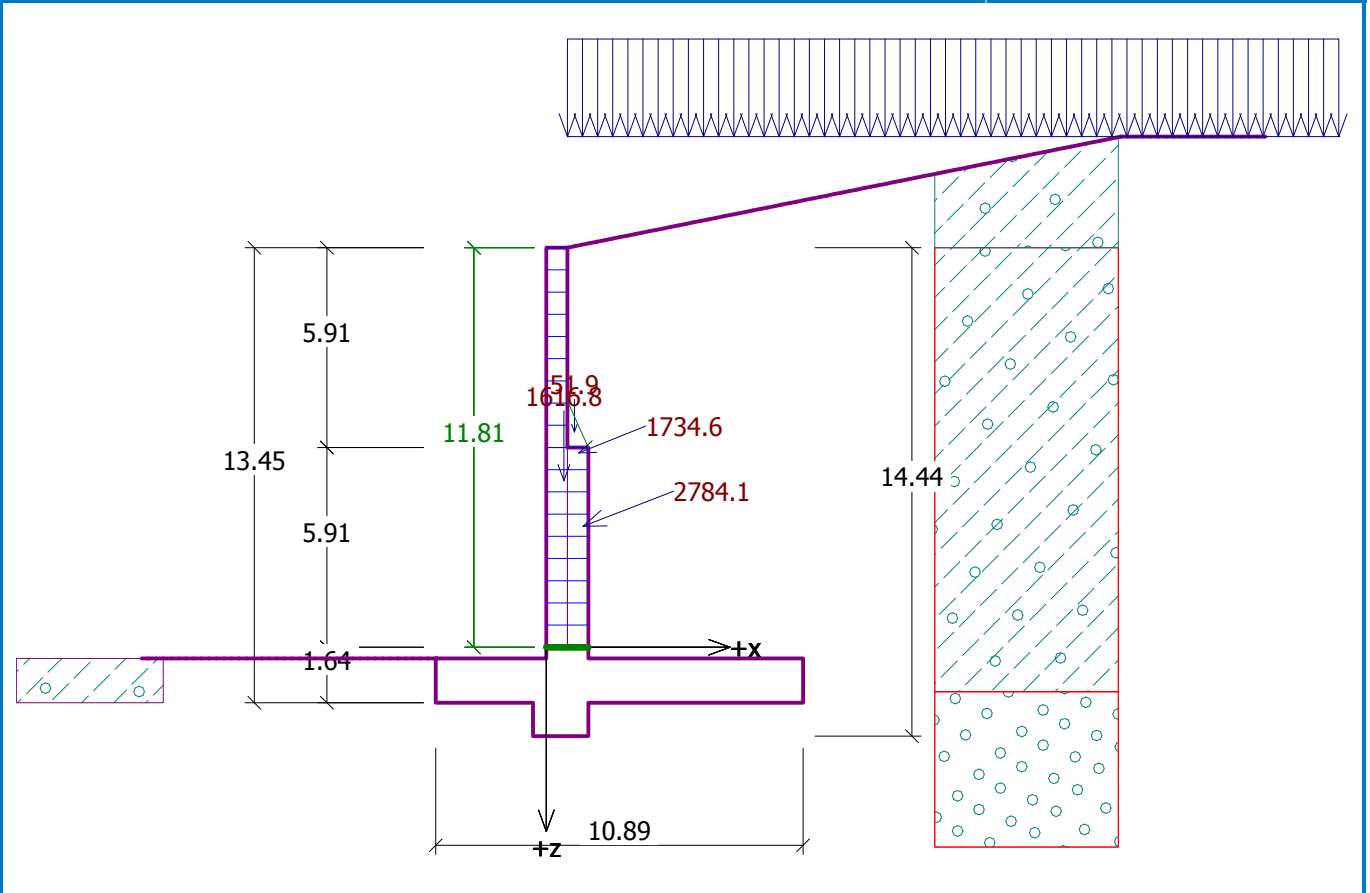
Verification of cross section in shear:

Ultimate shear force $V_{Rd} = 4443.9$ lbf/ft > 4207.5 lbf/ft = V_{Ed}

Cross section is SATISFACTORY.

Name : Dimensioning

Stage : 1; Dimensioning : 1



Dimensioning No. 2

Forces acting on construction

Name	F_{hor} [lb/ft]	App.Pt. Z [ft]	F_{vert} [lb/ft]	App.Pt. X [ft]	Design coefficient
Weight - wall	0.0	-2.62	479.0	0.31	1.000
Active pressure	326.1	-1.24	81.3	0.62	1.000
Surcharge No. 1	667.7	-2.45	179.9	0.62	1.000

Verification of the joint, 17.22 ft from the top.

Back face reinforcement :

Diameter = 0.63 in

Spacing = 11.81 in

Cover = 1.18 in

There is no reinforcement on the front face.

Wall aspect ratio: 9.47

Verification of cross section in compression:

Ultimate normal force $N_{Rd} = 120931.6 \text{ lbf/ft} > 740.2 \text{ lbf/ft} = N_{Ed}$

Cross section is SATISFACTORY

Verification of cross section in bending:

Ultimate bending moment $M_{Rd} = 7310.1 \text{ lbfft/ft} > 1962.0 \text{ lbfft/ft} = M_{Ed}$

Cross section is SATISFACTORY.

Verification of cross section in shear:

Ultimate shear force $V_{Rd} = 2221.9 \text{ lbf/ft} > 993.7 \text{ lbf/ft} = V_{Ed}$



Soil Boring - geotechnical services
Tiverton rd. 24
London, United Kingdom

tel:5478354125, fax:5478354126
www.soilboring.com, info@soilboring.com
Project: Shopping center - Black Rose

Cross section is SATISFACTORY.